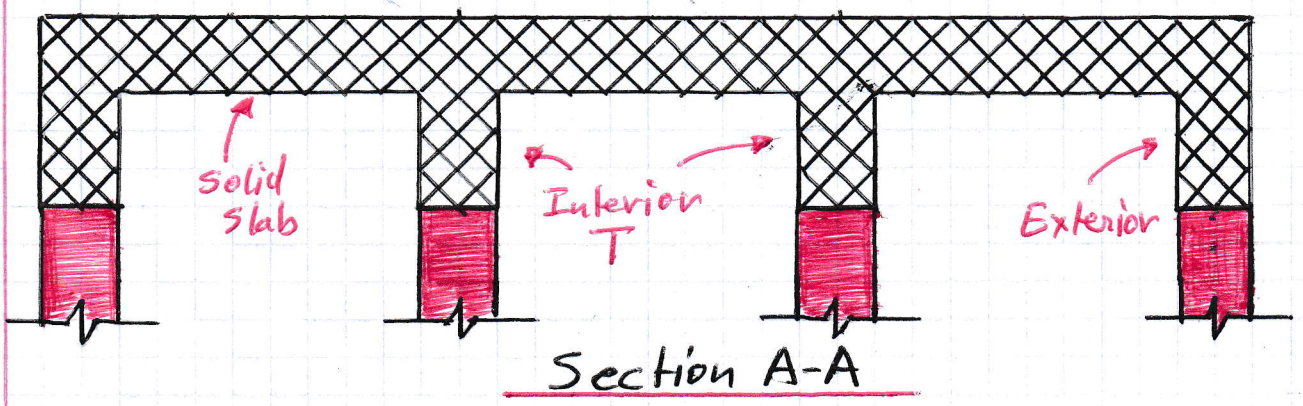
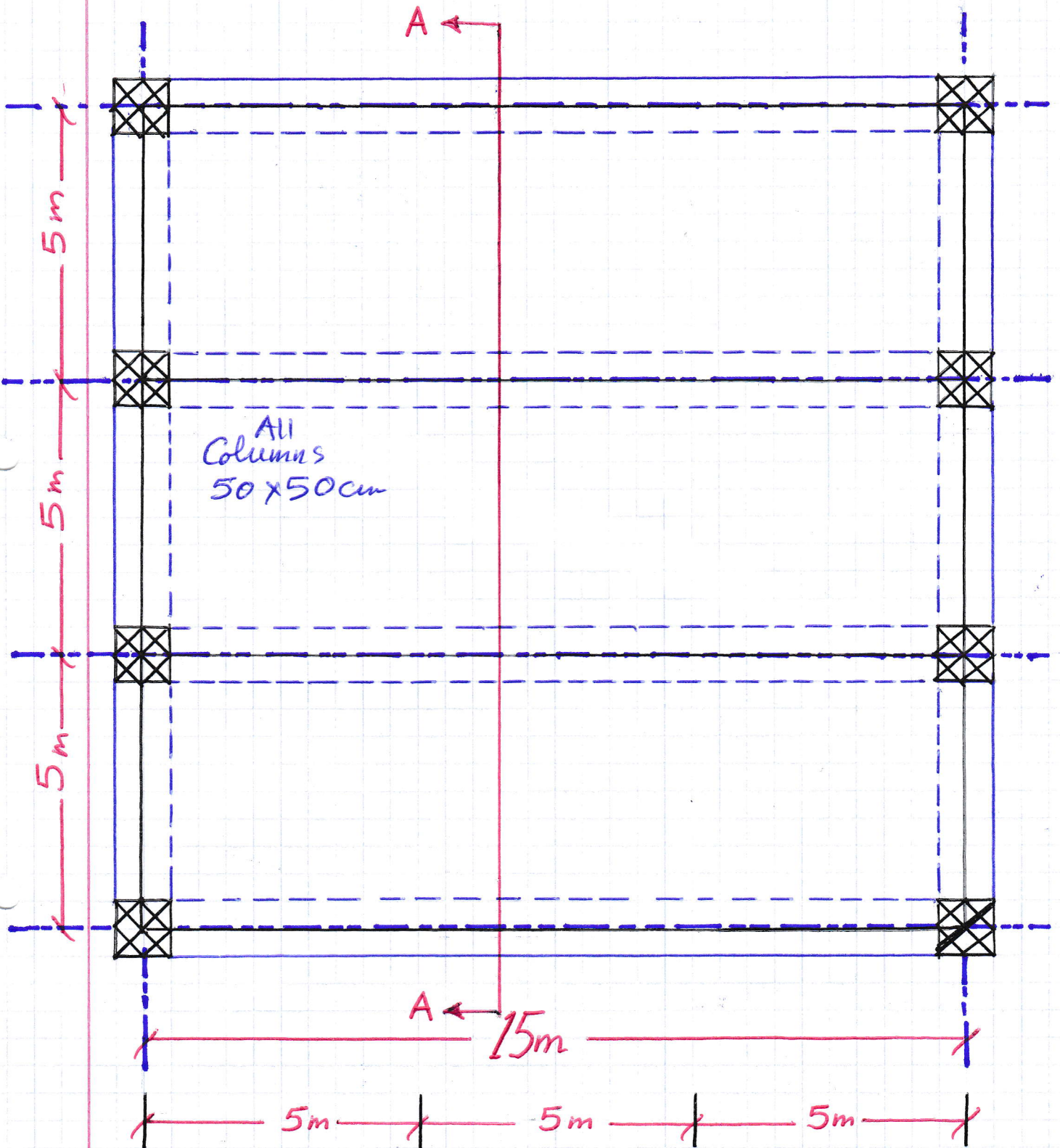


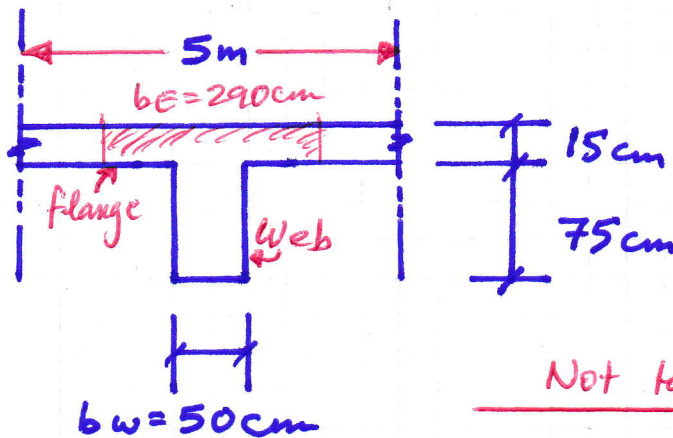
# T-Sections

①



Example :

(2)



$$f_c' = 28 \text{ MPa}$$

$$f_y = 420 \text{ MPa}$$

$$W_u = 2 \text{ t/m}^2$$

$\therefore W_u$  for an interior beam = 10 t/m  
(Tributary area)  
Same as the flange

$$M_u = \frac{(10)(15^2)}{8} = 281.2 \text{ t.m}$$

$$M_{nreq} = \frac{281.2}{0.9} = 312.5 \text{ t.m}$$

(must be checked)

$$b_E = 500 \text{ cm}$$

$$= 50 + \frac{1450}{4} = 412.5 \text{ cm}$$

$$= 50 + 16(15) = \underline{290 \text{ cm}} \text{ controls}$$

$a < t = 15 \text{ cm} ?$

if  $a = t$ ,  $C = 0.85(0.28)(290)(15) = 1035t$

$M_u = 1035 \left( \frac{83.5 - 15/2}{100} \right) = 787 \text{ t}\cdot\text{m}$

which significantly exceeds  $M_{u \text{ req}} = 312.5 \text{ t}\cdot\text{m}$

$\therefore$  it is clear that  $a < t_{\text{actual}}$

$\therefore$  Proceed as a rectangular section with  $b = 290 \text{ cm}$

$R_{u \text{ req}} = \frac{31250 \text{ t}\cdot\text{cm}}{(290)(83.5)^2} = 0.01546 \text{ t}/\text{cm}^2$

$P_{u \text{ req}} = 0.003808$

$A_{s \text{ req}} = 0.003808 \times 290 \times 83.5 = 92.21 \text{ cm}^2$

13.04  $\phi 30 \Rightarrow$  use 14  $\phi 30$  in two layers  
7  $\phi 30$  in each layer

$d_{\text{actual}} = 80.75 \text{ cm}$

Check capacity:

$A_{s \text{ actual}} = 98.98 \text{ cm}^2$ ,  $T = 415.7 \text{ t}$

$a = \frac{415.7}{0.85(0.28)(290)} = 6.02 \text{ cm}$

$M_u = 415.7 \left( \frac{80.75 - \frac{6.02}{2}}{100} \right) = 323.2 \text{ t}\cdot\text{m}$

$\phi M_u = 290.8 \text{ t}\cdot\text{m} > M_u = 281.2 \text{ t}\cdot\text{m}$

$\phi = 0.9 ?$

$$a = 6.02 \text{ cm},$$

(4)

$$x = \frac{6.02}{0.85} = 7.20 \text{ cm}$$

$$E_t = \frac{83.5 - 7.20}{7.20} (0.003) = 0.03179$$

$$\gg \gg 0.005$$

$$\therefore \phi = 0.9$$

$$A_{s \max} = ?$$

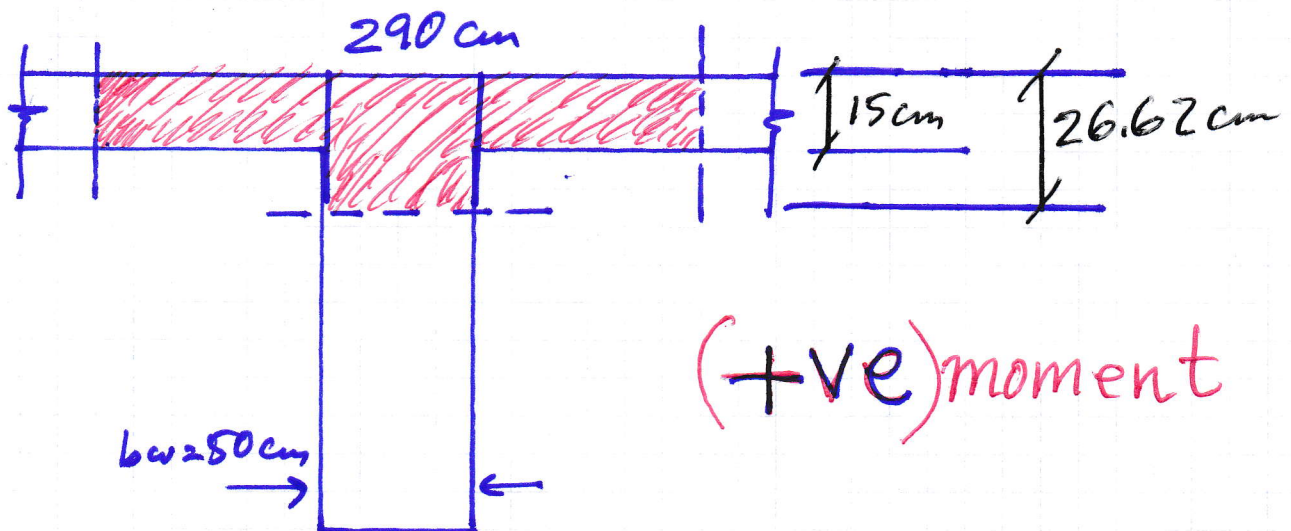
$$E_t = 0.005$$

$$x_{\max} = \frac{3}{8} (83.5) = 31.31 \text{ cm}$$

$$E_t = 0.005$$

$$a_{\max} = 26.62 \text{ cm} > t = 15 \text{ cm}$$

$$E_t = 0.005$$



(+ve) moment

$$C_{\max} = 0.85 (0.28) (290 - 50) (15)$$

$$E_t = 0.005$$

$$+ 0.85 (0.28) (50) (26.62)$$

$$= 1174 t$$

$$A_{s \max} = \frac{1174}{4.2} = 279.4 \text{ cm}^2$$

$$E_t = 0.005$$

(+ve)

$$\text{Note: } A_{s \text{ actual}} = 98.98 \text{ cm}^2$$

$$\therefore \phi = 0.9$$

For (-ve) moment, the section is actually rectangular with  $b = b_w = 50\text{cm}$  for our example, (5)

$$\text{and } A_{s\max} = 75,40\text{cm}^2$$

$E_t = 0,1005$   
(-ve)

$A_{s\min}$ , (+ve) moment  
look at the tension side of the beam,  
⇒ rectangular,

∴  $A_{s\min}$  same as before,

For our example,

$$= \frac{0,25\sqrt{28}}{420} (50)(83,5) \geq \frac{1,4}{420} (50)(83,5)$$
$$= 13,15\text{cm}^2 \geq \underline{13,92\text{cm}^2}$$

For (-ve) moment,

$$A_{s\min} = \frac{0,25\sqrt{f_c'}}{f_y} (bf)(dt) \geq \frac{1,4}{f_y} (bf)(dt)$$

larger  $f_y$

$$\frac{0,50\sqrt{f_c'}}{f_y} (b_w)(dt) \geq \frac{2,8}{f_y} (b_w)(dt)$$

larger  $f_y$

then  
Smaller

for our T-beam,

$$A_{s\min} \text{ (-ve)} = \frac{0,25\sqrt{28}}{420} (500)(83,5) \geq \frac{1,4}{420} (500)(83,5)$$

$131,5\text{cm}^2$   $139,2\text{cm}^2$

$$\frac{0,50\sqrt{28}}{420} (50)(83,5) \geq \frac{2,8}{420} (50)(83,5)$$

$26,3\text{cm}^2$   $27,8\text{cm}^2$  Controls