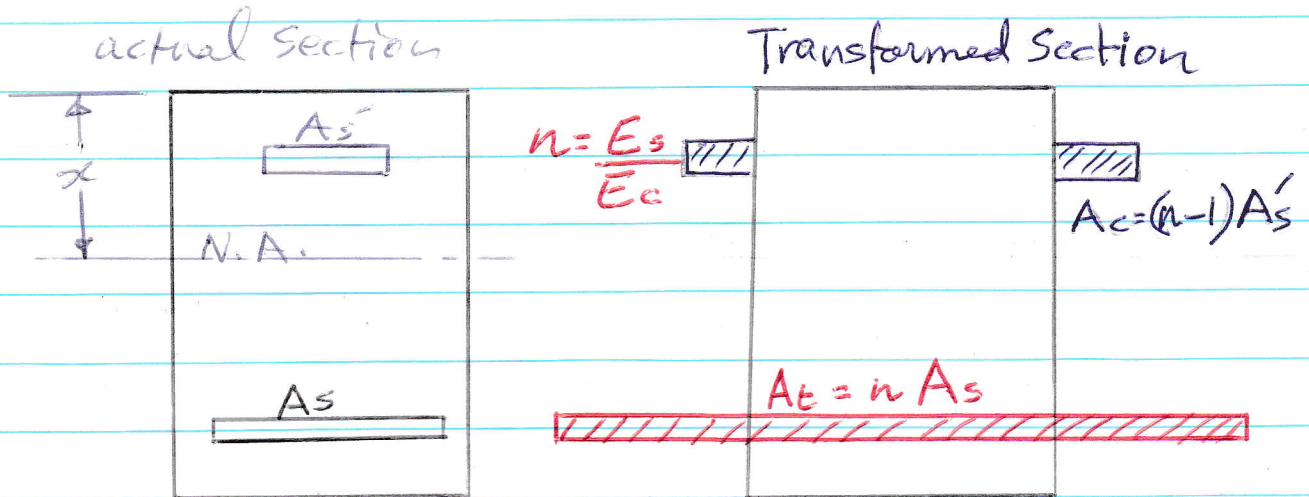


Deflections:

Transformed Section:



$$A_s f_s = A_t f_t$$

$$f_t = \frac{f_s}{n}$$

Example:

for $A_{s'} = 9.68 \text{ cm}^2$, $A_s = 45.68 \text{ cm}^2$

$f_c = 30 \text{ MPa}$, $E_s = 200\,000 \text{ MPa}$

$E_c = 4700 \sqrt{30} \approx 25\,000 \text{ MPa}$

$n = 8$, $b = 35 \text{ cm}$, $d = 55 \text{ cm}$, $d' = 6 \text{ cm}$
 $h = 61 \text{ cm}$

Locate N.A.

$A_t = 8(45.68) = 365 \text{ cm}^2$

$A_c = (8-1)(9.68) = 68 \text{ cm}^2$

$(35)(x)(\frac{x}{2}) + 68(x-6) = (365)(55-x)$

solving for x , $x = 24 \text{ cm}$

$I_{cr} = \frac{1}{3}(35)(24)^3 + 68(24-6)^2 + 365(55-24)^2 = 534,077 \text{ cm}^4$

I_g , ignoring steel, $= \frac{(35)(61)^3}{12} = 662,000 \text{ cm}^4$

Effective Moment of Inertia, I_e

$$I_e = \left(\frac{M_{cr}}{M_{max}} \right)^3 I_g + \left[1 - \left(\frac{M_{cr}}{M_{max}} \right)^3 \right] I_{cr} \leq I_g \quad (24.2.3.5 a)$$

$$M_{cr} = \text{cracking moment} = \frac{f_r I_g}{y_t} \quad (24.2.3.5 b)$$

$$f_r = 0.62 \sqrt{f_c}$$

y_t = distance from N.A. to extreme fiber of concrete in tension ($h/2$)

M_{max} = maximum service load-moment acting at the condition for which deflection is computed

a) Instantaneous deflection is the immediate (elastic) deflection determined using analysis.

b) Long-term (creep and shrinkage) deflection is determined as follows:

$$\Delta_{cptsh} = \lambda (\Delta i)_D$$

$$\lambda = \frac{\xi}{1 + 50 \rho'}$$

$$\xi = 2.0 \quad (\geq 5 \text{ years})$$

$$= 1.4 \quad (1 \text{ year})$$

$$= 1.2 \quad (6 \text{ months})$$

$$= 1.0 \quad (3 \text{ months})$$

$$\rho' = \frac{A_s'}{bd}$$

$(\Delta i)_D$ = instantaneous deflection due to all sustained loads (usually dead load).

Maximum permissible deflections are provided by the ACI-Code.

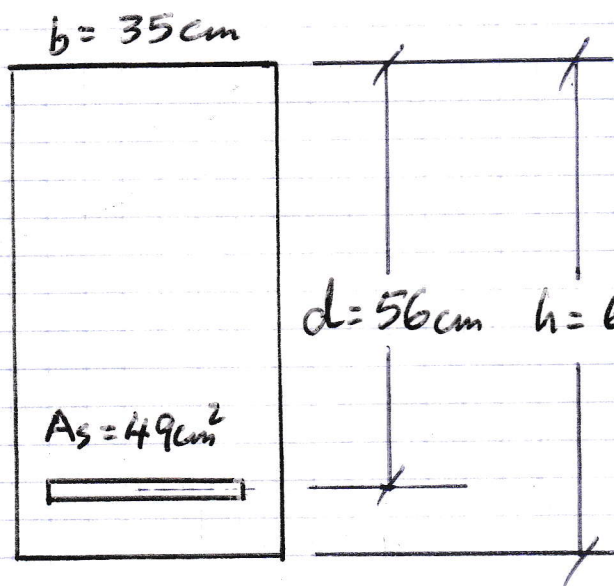
Example:

Investigate the deflection behavior for the beam shown. The beam is used on a 7.6-m simple span.

Service DL = 3.2 t/m, LL = 2.9 t/m

use $f'_c = 28 \text{ MPa}$, $f_y = 300 \text{ MPa}$

$E_c = 24870 \text{ MPa}$



The beam supports a floor with several partitions that will be damaged by large deflections.

$$M_{DL} = \frac{3.2(7.6^2)}{8} = 23.1 \text{ t.m}$$

$$M_{LL} = \frac{(2.9)(7.6)^2}{8} = 20.9 \text{ t.m}$$

From ACI-Table 9.3.1.1, for $f_y = 300 \text{ MPa}$,

$$h_{min} = \frac{l}{16} \left(0.4 + \frac{f_y}{700} \right) = \frac{760}{16} \left(0.4 + \frac{300}{700} \right) = 39.4 \text{ cm}$$

(Note that $h = 65 \text{ cm}$ is used, which considerably exceeds the specified 39.4 cm .)

$$W_u = 1.2(3.2) + 1.6(2.9) = 8.48 \text{ t/m}$$

$$M_u = \frac{(8.48)(7.6)^2}{8} = 61.22 \text{ t.m}$$

$$M_{n,req} = \frac{61.22}{0.9} = 68.03 \text{ t.m}$$

$$m = 12.61$$

$$R_{n,req} = \frac{6803}{(35)(56)^2} = 0.0620 \text{ t/cm}^2$$

$$p_{req} = 0.02443, A_{s,req} = 47.88 \text{ cm}^2, A_{s,actual} = 49 \text{ cm}^2$$

$$p_{max} = 0.31875 \left(\frac{28}{300} \right) (0.85) = 0.0253, A_{s,max} = 49.57 \text{ cm}^2$$

$$\frac{p_{req}}{p_{max}} = \frac{0.02443}{0.0253} = 0.96 \quad (\text{The required is very close to the maximum})$$

Although a depth of 65 cm is used, the high reinforcement ratio strongly indicates that a deflection problem is likely to occur.

$$I_g = \frac{(35)(65^3)}{12} = 800990 \text{ cm}^4$$

for the cracked section, locate the neutral axis under service loads,

$$n = \frac{200000 \text{ MPa}}{24870 \text{ MPa}} = 8.04$$

$$(35)(x)(x/2) = (49 \times 8.04)(56 - x)$$

$$\therefore x = 26.0 \text{ cm}$$

$$I_{cr} = \frac{1}{3}(35)(26.0)^3 + (49 \times 8.04)(56 - 26)^2 = 204970 \text{ cm}^4 + 354560 \text{ cm}^4$$

$$I_{cr} = 559530 \text{ cm}^4$$

Dr. Jamal Zlati (769)

$$M_{cr} = \frac{f_r I_g}{y_t} \quad (24.2.3.5b)$$

$$f_r = 0.627 \sqrt{f_c} \quad (19.2.3.1) = 3.28 \text{ MPa}$$

$$M_{cr} = \frac{(3.28 / 100) \text{ t/cm}^2 (800990 \text{ cm}^4)}{32.5 \text{ cm}} \times \frac{1 \text{ m}}{100 \text{ cm}}$$

$$= 8.09 \text{ t.m}$$

For DL deflection, $\frac{M_{cr}}{M_{max}} = \frac{8.09}{23.1} = 0.35$

$$(I_e)_{DL} = (0.35)^3 (800990) + [1 - (0.35)^3] (559530)$$

$$= 569880 \text{ cm}^4 < I_g$$

For total load deflection, $\frac{M_{cr}}{M_{max}} = \frac{8.09}{44} = 0.184$

$$(I_e)_{TL} = (0.184)^3 (800990) + [1 - (0.184)^3] (559530)$$

$$= 561030 \text{ cm}^4 < I_g$$

Immediate Deflections:

$$(DL): (\Delta_i)_{DL} = \frac{5 W l^4}{384 EI} = \frac{(5)(3.2)(7.6^4)(100^3)}{(384)(248.7)(569880)}$$

$$= 0.98 \text{ cm}$$

$$(TL): (\Delta_i)_{TL} = \frac{(5)(6.1)(7.6^4)(100^3)}{(384)(248.7)(561030)} = 1.90 \text{ cm}$$

$$\therefore (\Delta_i)_{LL} = 1.90 - 0.98 = 0.92 \text{ cm}$$

Long-term Deflections:

$$\Delta_{\Delta} = 2.0 / (1 + 50(\rho)) = 2.0$$

$$\Delta_{\text{long term}} = (2.0)(0.98) = 1.96 \text{ cm}$$

From Table 24.2.2, the maximum permissible calculated deflection,

$$(\Delta_i)_{LL} + (\Delta)_{\text{long term}} \leq l/480$$

$$0.92 + 1.96 = 2.88 \text{ cm}$$

$$l/480 = \frac{760}{480} = 1.58 \text{ cm}$$

which clearly indicates that a deflection problem exists as was expected due to the high reinforcement ratio.

Note that Table 9.3.1.1 requirements would not be sufficient to prevent deflection problems. The reinforcement ratio is a much better indicator.