

Design

Known Beam dimensions



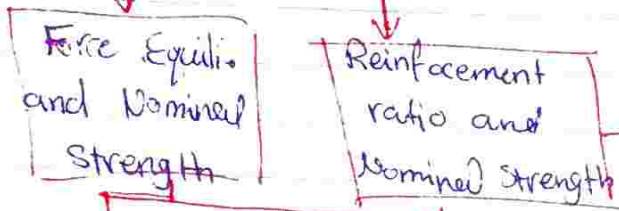
calculate the required reinforcement

Unknown Beam dimensions

known M_u
set the reinforcement ratio ρ, A, B, H

set the reinforcement ratio

Economic reinforcement $0.5 \rho_{max} \leq \rho \leq 0.75 \rho_{max}$



structural analysis $\frac{M_u}{\phi b d^2} = \rho f_y (1 - 0.59 \rho \frac{f_y}{f_c})$

$b d^2 = \square$

Find an adequate section (B, d)

$d = [1.5 \text{ to } 3] B$

ويعرفون بها 5cm

$a = \frac{A_s f_y}{0.85 f_c' B}$

$M_u = \phi M_n = \phi A_s f_y (d - \frac{a}{2})$

بجد لا يكون a كافي

assumption لا $a \approx d$

$a \approx \frac{d}{3}$

ويعرفون بها a

a - Checks

A_s \rightarrow $\rho b d$

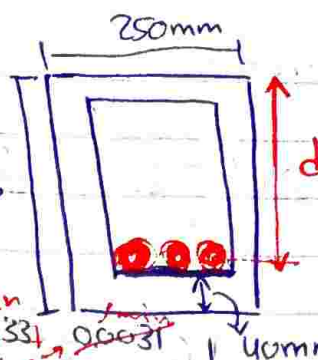
$A_s \rightarrow \rho b d$ \rightarrow $\rho b d$

الأولى

$M_u = \phi M_n = \phi \rho f_y B d^2 (1 - 0.59 \rho \frac{f_y}{f_c})$

Using design Aids on ITC

• Example: $M_u = 120 \text{ kN.m}$
 $f_c' = 28 \text{ MPa}$
 $F_y = 420 \text{ MPa}$



Design Aids: TABLE A.4

| | | | | |
|--------|-------|----------------|----------------|--------------|
| f_c' | B_1 | $\rho_{0.005}$ | $\rho_{0.004}$ | ρ_{min} |
| 28 | 0.85 | 0.0181 | 0.0206 | 0.00331 |

• y_d : $40 + 10 + 8 = 58 \text{ mm}$
 • $d = H - y_d = 500 - 58 = 442 \text{ mm}$

Assume $d = 16 \text{ mm}$
 • depth channel Strabs = 10mm
 • $\phi = 0.9$

① iterations: $a \rightarrow A_s \rightarrow \text{Check } a$
 \downarrow assume $= \frac{d}{3} = \frac{442}{3} \approx 147 \text{ mm}$

① $M_u = \phi A_s F_y (d - \frac{a}{2}) \Rightarrow 120 \times 10^6 = 0.9 A_s 420 (442 - \frac{147}{2})$
 $A_s = 853.3 \text{ mm}^2$

• check a : $a = \frac{A_s F_y}{0.85 f_c' B} = \frac{853.3 \times 420}{0.85 \times 28 \times 250} = 60.2 \text{ mm}$

② $a = 80 \text{ mm}$
 $M_u = \phi A_s F_y (d - \frac{a}{2}) \Rightarrow A_s = 789.7 \text{ mm}^2$

• check a : $a = \frac{A_s F_y}{0.85 f_c' B} = 55.7 \text{ mm}$

③ $a = 55 \text{ mm}$
 $A_s = 766 \text{ mm}^2 \rightarrow$ $a = 54 \text{ mm}$

بمعقد هاي
 لهان قربة ع الفرض عن هيك مقبل
 0.03 مقبل فرق

بهاد، مثال، الفرض زي
 هعنا، اولكن عا
 عا يزيد الليوسنة
 اوي بعد، وبعجل
 Check \rightarrow ρ_{req}


$A_s = 766 \rightarrow$ Design Aids (Table A.2)

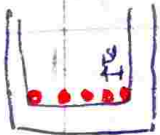
بشوف اذا هاي الباترات بتوسع باليم

| | |
|----|----|
| 24 | 22 |
| 34 | 16 |
| 36 | 13 |

أقرب قيم \leftarrow

• options:

$\Rightarrow 2\phi 22$ ✓ $\Rightarrow \rho = 250 - 20 - 44 - 80 = 106 > 25 \text{ mm}$
spaces 

$\Rightarrow 4\phi 16$ ✓ $\Rightarrow 3\rho \Rightarrow 250 = 80 + 20 + 4 \times 16 + 3\rho$
 $\rho = 28.6 > 25 \text{ mm}$ 

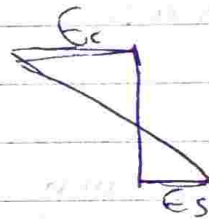
$\Rightarrow 6\phi 13$ ✗ $\Rightarrow 5\rho \Rightarrow 250 = 80 + 20 + 6 \times 13 + 5\rho$
 $\rho = 14.4 < 25 \text{ mm}$

Design Aids
 ↪ Table A.7

بدل ما يمكنه ادرج في

• Check on ϕ :

$0.004 < \rho < 0.005$



$\rho = \frac{A_s}{Bd} = \frac{796}{250 \times 442} = 0.0072 < \rho_{0.005}$ ✓
Table A.2
↪ ρ_max
> ρ_min

• Check on ϕM_n :

$a = \frac{A_s f_y}{0.85 f'_c b} = \frac{796 \times 420}{0.85 \times 28 \times 442} = 56.18 \text{ mm}$

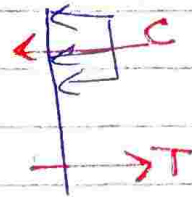
$\phi M_n = \phi A_s f_y \left(d - \frac{a}{2} \right)$
 $= 0.9 \times 796 \times 420 \left(442 - \frac{56.18}{2} \right)$
 $= 124.5 \text{ kNm} > M_u$ ✓

Note: $A_s \neq n \times \frac{\pi}{4} (d)^2$

لا نه يكون في ضربات في البارز عبا نرغ عتبه ابا صه للبارز
 القيم الي تبكون بالمباطل تبكون مراعية لها دالفرقه (الفرقه صغيره)

* Method 2:

→ $T = C$



- $a = \frac{A_s f_y}{0.85 f_c' B}$
- $\phi M_n = \phi A_s f_y (d - \frac{a}{2})$

→ $M_u \leq \phi M_n = \phi \rho f_y b d^2 (1 - 0.59 \rho \frac{f_y}{f_c'})$

$R = 120 \times 10^6$
 → $\frac{M_u}{\phi b d^2} = 2.73 \text{ MPa}$
 الطريقة الأتية

TABLE A.4

| ρ | $\rho_{0.005}$ $\rho_{0.005}$ | $\rho_{0.004}$ $\rho_{0.004}$ | $\rho_{min} = \frac{1.4}{f_y}$ | $\rho_{mi} = \frac{0.95 \sqrt{f_c'}}{f_y}$ |
|--------|----------------------------------|----------------------------------|--------------------------------|--|
| 0.25 | 0.181 | 0.0206 | 0.0033 | 0.0031 |

نظرات التي صارت فيها
 يعني متوسطي
 أخذت بوسطها عليها
 Table A.5a

| ρ | $f_y = 420 \text{ MPa}$ $f_c' = 28$ |
|--------|--|
| 0.0065 | 2.57 |
| 0.007 | 2.76 |

• $\frac{2.76 - 2.57}{0.007 - 0.0065} = \frac{2.73 - 2.57}{x - 0.0065}$
 $x = \rho = 0.00692 > \rho_{min}$
 $\rho < \rho_{max}$

بعد انتر بولينج
 للقمة

- $A_s = \rho d b = 764.7 \text{ mm}^2$
- $4 \phi 15 \rightarrow A_s = 796 \text{ mm}^2$
- $\rho = 0.0072$

TABLE A.5a

| ρ | $f_y = 420 \text{ MPa}$ $f_c' = 28$ |
|--------|--|
| 0.007 | 2.76 |
| 0.0075 | 2.94 |

$R = 2.844 \text{ MPa}$ مع الانتر بولينج

بزوج في الجباريل
 وينطول الباربات قطرها
 TABLE A.7 ← ρ

$R = \phi M_u = 2.844 \times 9 \times 250 \times (442)^2 = 125 \text{ KB.m}$
 $\phi b d^2$

$\phi M_n = 125 > M_u \text{ OK}$

→ Example: simply supported Beam

span → 4.5 m

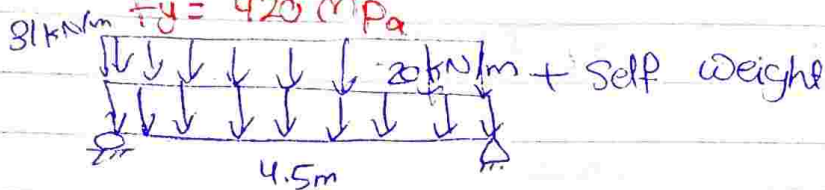
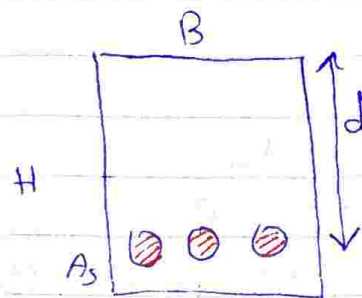
dead load → 20 kN/m

live load → 31 kN/m

$$\rho = 0.5 \rho_{max}$$

$$f_c = 28 \text{ MPa}$$

$$f_y = 420 \text{ MPa}$$



1) $M_u = ?$

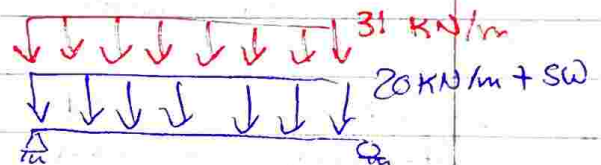
assume

initial assumptions.

• Table 9.3.1.1 Minimum depth $(H) = l/16 = 4.5/16 = 0.3 \text{ m} = 300 \text{ mm}$

$B = 200 \text{ mm}$

• $SW = 1 \times 0.3 \times 0.2 \times \gamma_c = 1.44 = 1.5$
 24 kN/m^3



• $W_u = 1.2 D + 1.6 L$
 $= 1.2(21.5) + 1.6(31)$
 $= 75.4 \text{ kN/m}$

• Simply supported Beam → $M_u = \frac{W L^2}{8} = \frac{(75.4)(4.5)^2}{8} = 191 \text{ kN.m}$

• $\rho = 0.5 \rho_{max} = 0.5 \times 0.0206 = 0.0103$

TABLE A.4

Method 2 : $\rho = 0.0103 \rightarrow R = ??$

| ρ | R |
|--------|------|
| 0.01 | 3.83 |
| 0.0105 | 4 |

$R = 3.93 \text{ MPa}$

$R = \frac{M_u}{bd^2} \Rightarrow bd^2 = \frac{191 \text{ kN.m}}{3.93} = 48.3 \times 10^6 \text{ mm}^3$

ϕbd^2

↳ If $\rho < \rho_{0.005} \Rightarrow \phi = 0.9$

- mostly economical $\rightarrow d = (1.5-3)b$

ب. d تكون $> 1.5b$ \leftarrow من المبدأ
 يجب ما يوسع لباررات مناسبة
 $d > b$

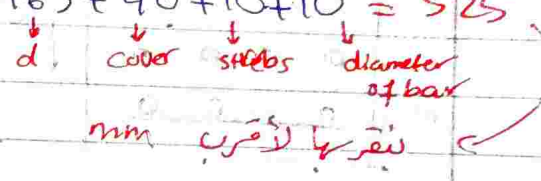
| b mm | d mm | factor |
|------|------|--------|
| 200 | 520 | 2.6 |
| 300 | 425 | 1.5 |
| 400 | 380 | 0.95 |
| 250 | 465 | 1.86 |

250 \leftarrow yield

o B \rightarrow 250 mm

o d \Rightarrow 465 mm $\rightarrow H = 465 + 40 + 10 + 10 = 525$

o Check S.W =



set to $\Rightarrow B = 250$ mm

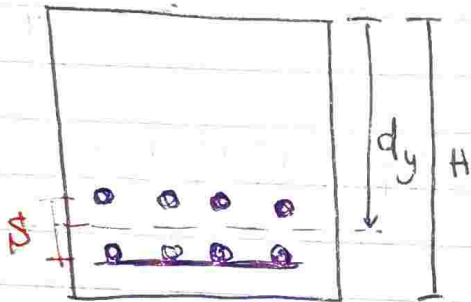
$H = 550$ mm

$S.W = 3.4$ kN/m

o $M_u = \frac{(77.7) * (4.5)^2}{8} = 197$ kN.m

$\rightarrow 1.2(23.4) + 1.6(31)$

- positive moment \rightarrow Bottom reinforcement
- negative moment \rightarrow Top reinforcement



مع \rightarrow reinforcement

$$d_y = H - \left(\text{cover} + \text{strabs} + d + \frac{1}{2} \phi \right)$$