Ch.11 : Displacement Method

{force Method}
flexibility Method

deformation due to unit load

{Displacemend method}
Stiffness Method

force due to unit deformation

- Displacement method:

simplifications
- slope - deflection equations
- Moment distribution method

} flextural systems

- General procedure:-

Direct Stiffness Method $\{F\} = [K]\{D\}$

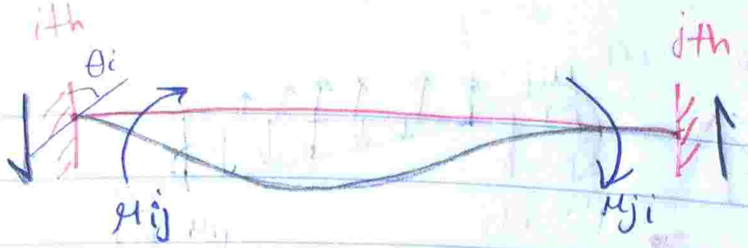
Finit Element Method

- Slope - deflection Equations:-

→ to derive stiffness cofficients (Beam element)

Kinematically det.

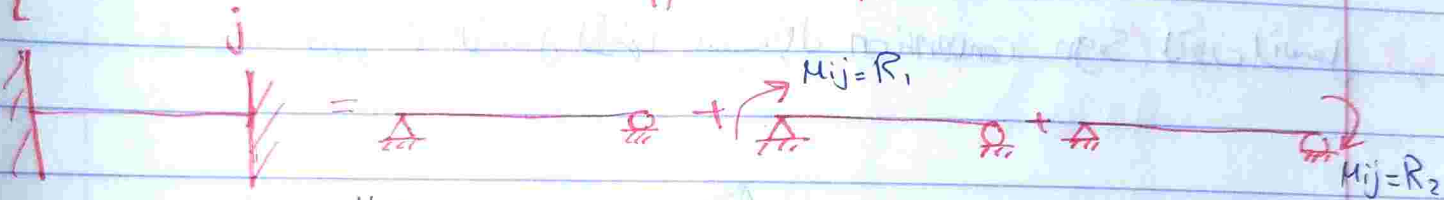
<force method>
- Static indet.:- unknown forces
- Kinematic indet. :- unknown deformations
<deformation method>



- M_{ij} , M_{ji} :- end-moment.

  - $M_{ij} = \boxed{}\ \theta_i$
  - $M_{ji} = \boxed{}\ \theta_i$

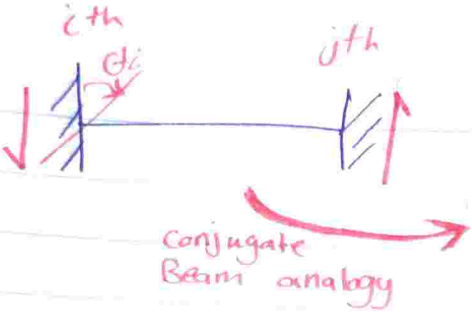
  ↳ steffness coefficients.

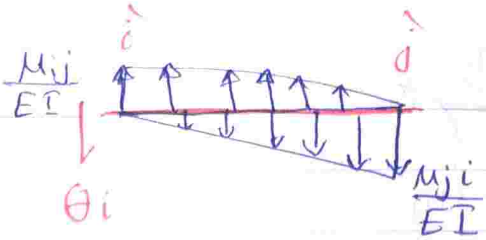


- rotational Sett at i^{th} θ_i Gω
- compatability Equ. at i : $\quad = D_{10} + R_1 d_{11} + R_2 d_{12}$

- compatabilit Equ. at j : $\quad 0 = D_{20} + R_1 d_{21} + R_2 d_{22}$

or



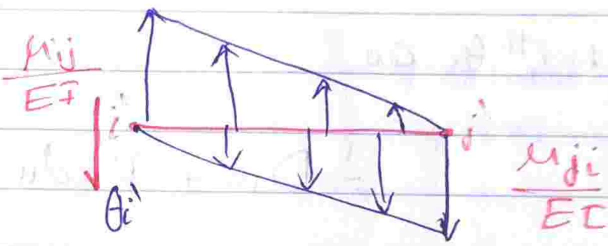


Real Beam — Conjugate Beam

→ θ C.C.W — Shear (+)

→ Δ ↑ — Moment (+)

CW نضع θ_i للأسفل لأنها حسب ال (Sign convention) تكون للأسفل لأنها

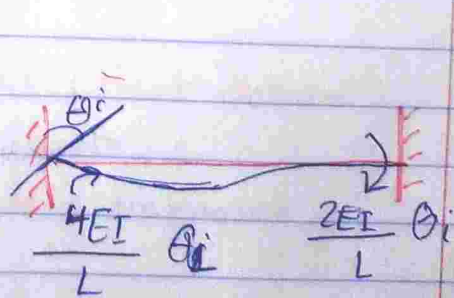


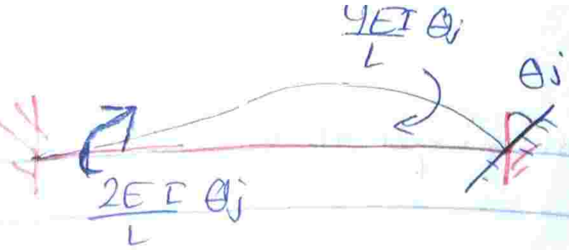
- $\Sigma M_i = 0 = \frac{1}{2}\left(\frac{M_{ij}}{EI}\right)(L)\left(\frac{L}{3}\right) - \frac{1}{2}\left(\frac{M_{ji}}{EI}\right)L\left(\frac{2}{3}L\right)$

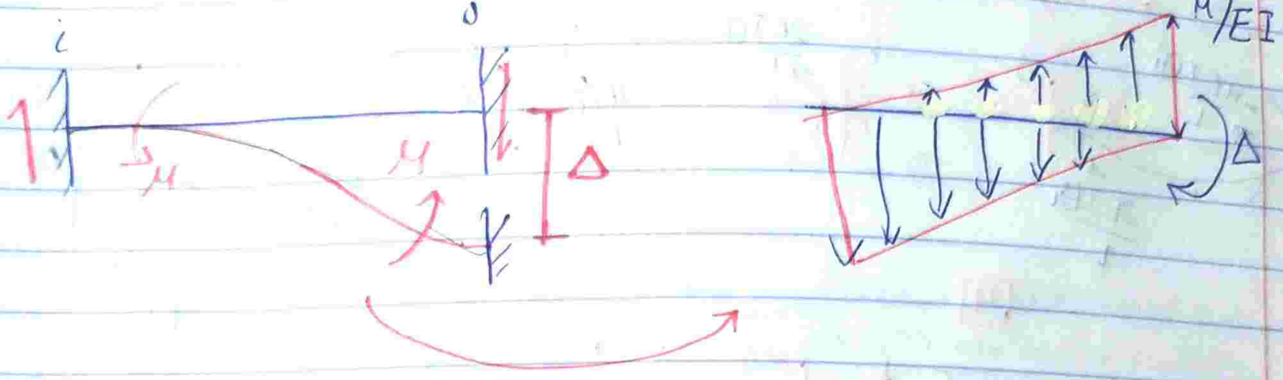
- $M_{ji} = \frac{1}{2} M_{ij}$

- $M_j' = 0 = \frac{1}{2}\left(\frac{M_{ij}}{EI}\right)(L)\left(\frac{2}{3}L\right) - \frac{1}{2}\left(\frac{M_{ji}}{EI}\right)(L)\left(\frac{L}{3}\right) - \theta_i(L)$

- $M_{ij} = \frac{4EI}{L}\theta_i$
- $M_{ji} = \frac{2EI}{L}\theta_i$

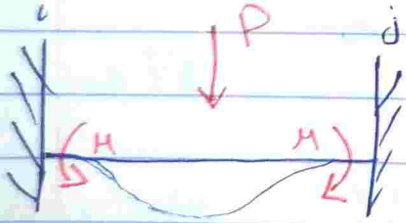


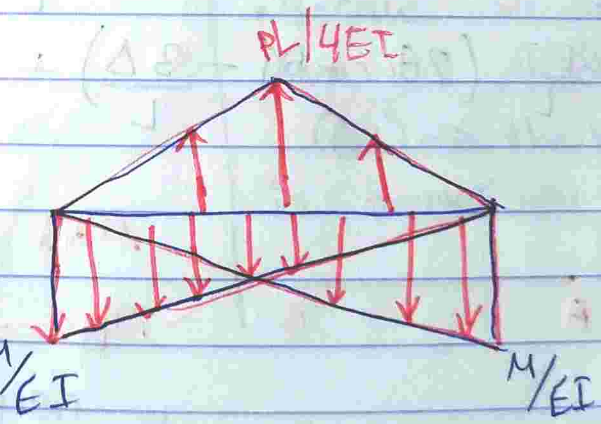




Conjugat Beam anology

- $\Sigma M_j' = 0$
- $M = \dfrac{6EI}{L^2}\Delta$





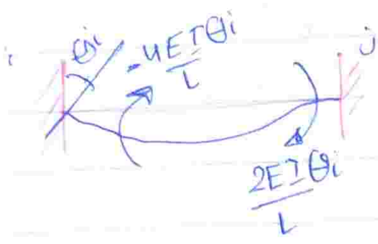
$\Sigma F_y = 0$

$M = PL/8$

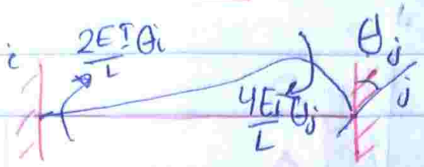
$\rightarrow M_{ij} = (\quad)\theta_i + (\quad)\theta_i + (\quad)\Delta +$ Moment applied load

Slope-deflection Equations

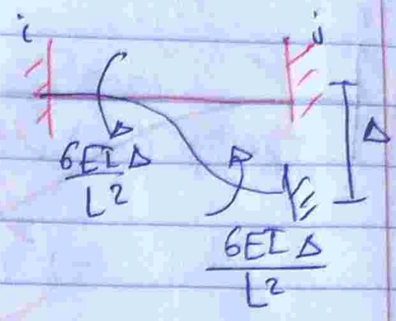
[1]



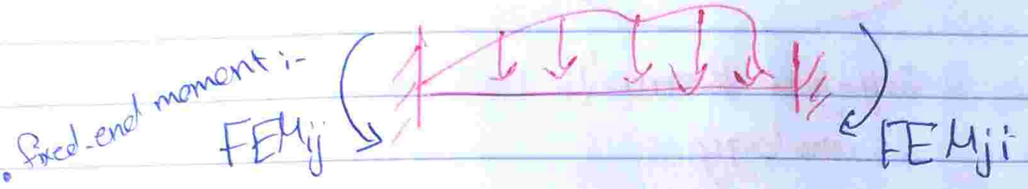
[2]



[3]



[4]



⇒ Equation for the End moment M_{ij}

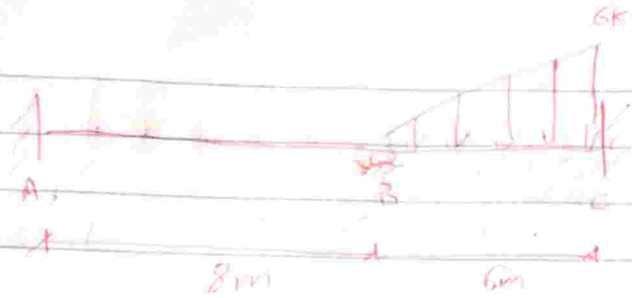
CW +
CCW −

$$M_{ij} = \frac{4EI}{L}\theta_i + \frac{2EI}{L}\theta_j - \frac{6EI}{L^2}\Delta + FEM_{ij}$$

$$M_{ij} = \frac{2EI}{L}\left(2\theta_i + \theta_j - \frac{3\Delta}{L}\right) + FEM_{ij}$$

$M_{AB} \Rightarrow i = A,\ j = B$

$M_{BA} \Rightarrow i = B,\ j = A$



- Draw BM diagram indicating key values
- Sketch Deformed shape

① divide the system into discrete element

• we have two elements AB + BC

② write end-moment equations

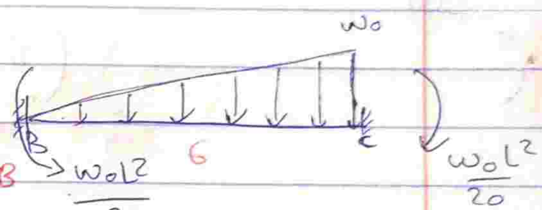
zero → fixed support   zero → No diffrential settlement   No applied loading

• $M_{AB} = \frac{2EI}{8}\left(\cancel{2\theta_A} + \theta_B - \cancel{\frac{3\Delta_{AB}}{8}}\right) + \cancel{FEM_{AB}}^{\,0}$

• $M_{BA} = \frac{2EI}{8}\left(2\theta_B + \cancel{\theta_A}^{\,0} - \cancel{\frac{3\Delta_{AB}}{8}}^{\,0}\right) + \cancel{FEM_{BA}}^{\,0}$

• $M_{BC} = \frac{2EI}{6}\left(2\theta_B + \cancel{\theta_C}^{\,0} - \cancel{\frac{3\Delta_{BC}}{6}}^{\,0}\right) + FEM_{BC}$

• $M_{CB} = \frac{2EI}{6}\left(\cancel{2\theta_C}^{\,0} + \theta_B - \cancel{\frac{3\Delta_{BC}}{6}}^{\,0}\right) + FEM_{CB}$

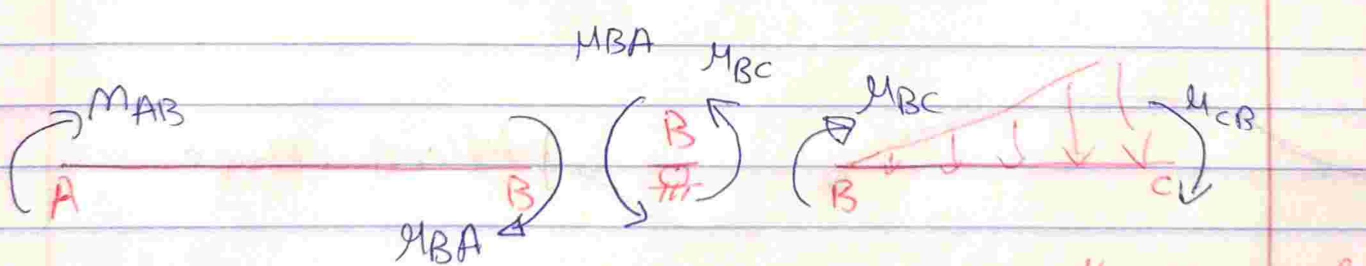


$\rightarrow FEM_{BC} = - \frac{6(6)^2}{30} =$

$\rightarrow FE\,M_{CB} = + \frac{6(6)^2}{20} =$

• Becaus we don't knew 1 deformation (θ_B) → the system is kinmetically Ind.

⇒ Solve for $\boxed{\theta_B}$

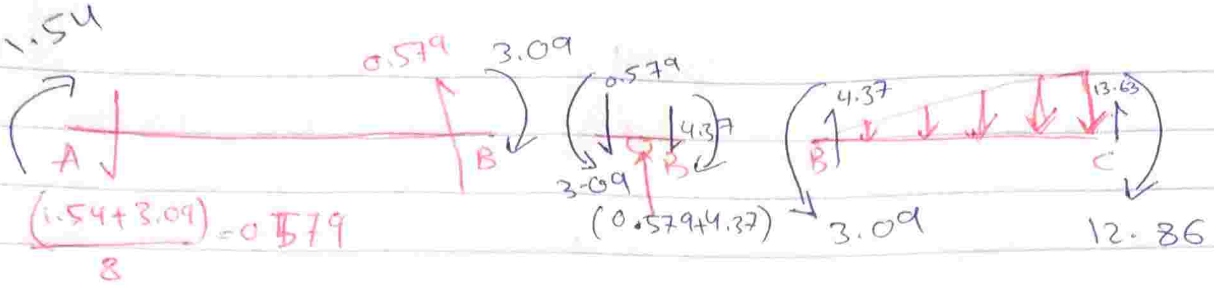


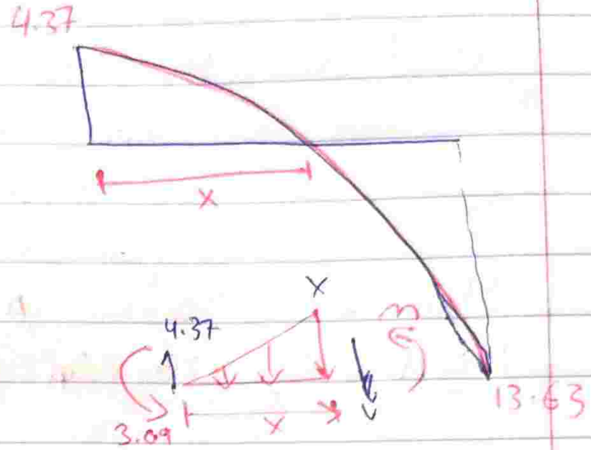
• $\Sigma M_B = 0 \Rightarrow M_{BA} + M_{BC} = 0$ → at the connected joint

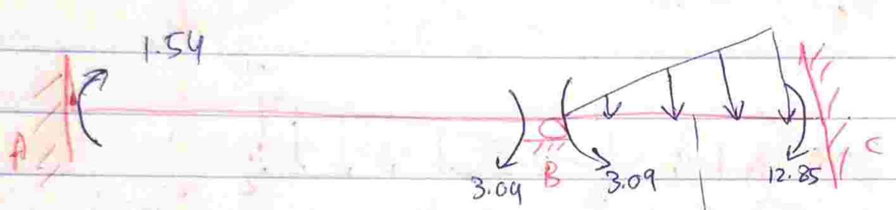
$\frac{4EI}{8}\theta_B + \frac{4EI}{6}\theta_B - 7.2 = 0$

$\theta_B = 6.17/EI$

• $M_{AB} = 1.54$ kN.m • $M_{BA} = 3.04$ kN.m • $M_{BC} = -3.08$ • $M_{CB} = 12.85$







$\cdot \Sigma F_y = 0$

$4.37 - \frac{1}{2}(x)(x) = 0$

$4.37 = x^2/2$

$x = 2.96\ m$

$\cdot \Sigma M_{cut} = 0$

$M = 5.5$