

Soil Mechanics

Midterm Exam

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Q1: ① from the graph: $D_{10} = 0.13$ mm

② $D_{30} = 0.32$

$D_{60} = 0.9$

$$C_u = \frac{D_{60}}{D_{10}} = \frac{0.9}{0.13} = 6.923$$

$$C_c = \frac{(D_{30})^2}{D_{10} D_{60}} = \frac{(0.32)^2}{(0.13)(0.9)} = 0.875$$

③ according to USCS: Gravel = 4%

Sand = 91.4%

finer = 4.6%

④ AASHTO: Percent finer on No. 200 = 4.6% $\leq 35\%$

$LL = 10$, $PL = 5$, $PI = 5$

A-1-b(0)

GI \swarrow

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~~USCS~~ USCS: Gravel = 4% < 15%

Sand = 91.4% > 50%

Fines = 4.6%

LL = 10 / PI = 5

$4 \leq PI \leq 7$

SC - SM

(silty clayey sand)

$$\text{Q2:-) } \textcircled{1} V_{\text{compacted fill}} = 1750 \text{ m}^3$$

$$\gamma_B(\text{compacted fill}) = 18 \text{ KN/m}^3$$

$$w = 8\%$$

$$\gamma_d(\text{compacted fill}) = \frac{\gamma_B}{1+w} = \frac{18}{1.08} = 16.667 \text{ KN/m}^3$$

* Option ①: $\gamma_d = 15 \text{ KN/m}^3$, $G_s = 2.7$, $w = 5\%$

$$\gamma_d V = \gamma_d(\text{fill}) V_{\text{fill}}$$

$$V = ~~1940~~ 1944.483 \text{ m}^3$$

$$\text{no. of trucks} = \frac{1944.483}{15} = 129.6 \text{ trucks}$$

$$\text{Cost} = (129.6)(375) + 10(1944.483)$$

$$= 48600 + 19444.83$$

$$= 68044.83 \text{ NIS}$$

* option ②: $\gamma_d = 13 \text{ KN/m}^3$, $G_s = 2.7$, $w = 7\%$

$$\gamma_d V = \gamma_d(\text{fill}) V_{\text{fill}} \rightarrow V = 2243.635 \text{ m}^3$$

$$\text{no. of trucks} = 149.576 \text{ trucks}$$

$$\text{Cost} = (149.576)(300) + (22436.35)$$

$$= 67309.15 \text{ NIS}$$

* option (2) is more economical

(2) for compacted fill:

$$\gamma_d = 16.67 \text{ kN/m}^3$$

$$\begin{aligned} * \gamma_d &= \frac{G_s \gamma_w}{1+e} \quad \rightarrow \quad e = \frac{G_s \gamma_w}{\gamma_d} - 1 \\ &= 0.59 \end{aligned}$$

$$* S_e = G_s w$$

$$S = 36.6 \%$$

$$* n = \frac{e}{1+e} = 0.371$$

Q 3:-) ① optimum moisture content = 15.6 %

② Maximum dry density = 1.642 g/cm^3

③ $\gamma_{\text{max}} = \gamma_{d_{\text{max}}} (1 + w)$
 $= 1.642 (1 + 0.156)$
 $= 1.898 \text{ g/cm}^3$

④ $\text{Doc} = \frac{\gamma_d(\text{field})}{\gamma_d(\text{max})}$

$\gamma_d(\text{field}) = 0.97 * (1.642)$
 $= 1.593 \text{ g/cm}^3$

$$\textcircled{Q} 4:-) \text{ At point A: } K_E = 2 \times 10^{-3} \text{ cm/sec}$$

$$\text{At point B: } K_E = \frac{K_2 H_2 + K_3 H_3}{H_2 + H_3}$$

$$= \frac{1 \times 10^{-3} \times 5 + \frac{2}{3} \times 10^{-3} \times 5}{10}$$

$$= 0.833 \times 10^{-3} \text{ cm/sec}$$

$$\text{At point C: } K_E = \frac{H_4 + H_1}{\frac{H_4}{K_4} + \frac{H_1}{K_1}}$$

$$= \frac{25}{\frac{2 \times 12.5}{2 \times 10^{-3}}}$$

$$= 2 \times 10^{-3} \text{ cm/sec}$$

$$\text{for all soils: } K_E = \frac{K_1 H_1 + K_2 H_2 + K_3 H_3}{H_1 + H_2 + H_3}$$

$$= 1.75 \times 10^{-3} \text{ cm/sec}$$

$$\text{At point A: total head} = 55 + 20 + 10$$

$$= 85 \text{ cm}$$

$$\text{elevation head} = 30 \text{ cm}$$

$$\text{pore water pressure} = \gamma_w h_w$$

$$= 9.81 \times \frac{55}{100}$$

$$= 5.4 \text{ kN/m}^2$$

$$\text{At point B: total head} = 30 + 10 = 40 \text{ cm}$$

$$\text{elevation head} = 10 \text{ cm}$$

$$\text{pore water pressure} = \gamma_w h_w$$

$$= 9.81 \left(\frac{30}{100} \right)$$

$$= 2.943 \text{ kN/m}^2$$

$$\text{At point C: total head} = (85 - 55 - 10) + 30 + 10$$

$$= 60 \text{ cm}$$

$$\text{elevation head} = 40 \text{ cm}$$

$$\text{pore water pressure} = 9.81 \left(\frac{20}{100} \right)$$

$$= 1.962 \text{ kN/m}^2$$