

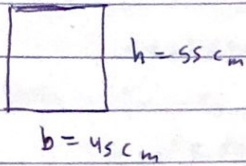
- Homework - Slender Columns - ENCE 436

HW #4 - Mohamad Moayad Shannaq - 1181401.

Beams

5 @ 6m = 30m

b = 45 cm, h = 55 cm

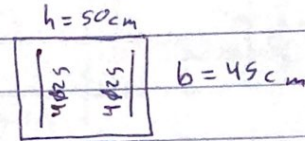


Columns

5 @ 8m = 40m

b = 45 cm, h = 50 cm

4 ϕ 25 along each end face.



"braced frame".

$f_c' = 28 \text{ MPa}$, $f_s = 420 \text{ MPa}$.

Service loads

Dead

$P = 200 \text{ t}$

$M_2 = 3 \text{ t.m}$

$M_1 = -3 \text{ t.m}$.

Live

$P = 100 \text{ t}$

$M_2 = 45 \text{ t.m}$

$M_1 = 17 \text{ t.m}$.

* Check the capacity of this interior third floor column.
Show that under the magnified eccentricity
 $\phi P_n \geq P_u$.

Solution:

$$L_c = 8 \text{ m}, \quad L_u = 8 - 0.55 = 7.45 \text{ m}$$

$$L_g = 6 \text{ m}$$

$$P_u = 1.2(200) + 1.6(100) = 400 \text{ t}$$

$$M_{u2} = M_2 = 75.6 \text{ t.m}$$

$$M_1 = 1.2(-3) + 1.6(17) = 23.6 \text{ t.m}$$

} Single Curvature.

$$e_{\text{actual}} = 18.9 \text{ cm}$$

$$\psi_A = \psi_B = \frac{\sum [E_c I_c / L_c]}{\sum [E_g I_g / L_g]}$$

$$; I_c = 0.7 I_{g \text{ gross column}}$$

$$I_g = 0.35 I_{g \text{ gross beam}}$$

$$I_c = (0.7) \frac{1}{12} BH^3 = 328125 \text{ cm}^4$$

$$I_g = (0.35) \frac{1}{12} BH^3 = 218367 \text{ cm}^4$$

$$E_c = E_g$$

$$\Rightarrow \psi_A = \psi_B = \frac{(2) [(328125) / 800]}{(2) [(218367) / 600]} = 1.127 \xrightarrow{\text{alignment chart}} K = 0.78$$

- check slenderness effects :

$$r = 0.3 h$$

$$\frac{K L_u}{r} = \frac{(0.78)(7.45)}{(0.3)(0.5)} = 38.74$$

$$\left(\frac{K L_u}{r} \right)_{\text{limit}} = \min \left\{ 34 + 12 \left(\frac{M_{1ns}}{M_{2ns}} \right) = 34 - 12 \left(\frac{23.6}{75.6} \right) = 30.25 \right. \\ \left. 40 \right.$$

\(\therefore\) So slenderness effects must be considered.

$$M_{\text{max}} = \left(\frac{C_m}{1-\alpha} \right) M_m$$

$$\rightarrow C_m = 0.6 - 0.4 \frac{M_{1ns}}{M_{2ns}} \geq 0.4$$

$$= 0.6 - 0.4(-0.312) = 0.725 \geq 0.4$$

$$\rightarrow \alpha = P_u / 0.75 P_{cr}$$

$$\rightarrow P_{cr} = \frac{\pi^2 EI}{(K L_u)^2}$$

$$EI = \text{largest of } \left\{ \frac{0.12 E_c I_g + E_s I_{sc}}{1 + B_{dhs}}, \frac{0.4 E_c I_g}{1 + B_{dhs}} \right.$$

$$M_{m, \text{min}} = P_u (e_{\text{min}}), \quad e_{\text{min}} = 15 + 0.103 h$$

$$= (400)(15 + 0.103(500)) = 12 \text{ t.m.} \\ = 12000 \text{ t.mm.} < 75.7$$

M_{min} not controlled.

$$\rightarrow B_{dhs} = \frac{1.2 D_L}{P_u} = 0.6$$

$$\Rightarrow E_{\text{concrete}} = 4700 \sqrt{f_c'} = 24\,870 \text{ MPa} = 248.7 \text{ t/cm}^2$$

$$I_{\text{gross}} = \frac{1}{12} BH^3 = \frac{1}{12} (45)(50)^3 = 470\,000 \text{ cm}^4$$

$$A_{\text{bar}} = 4.9 \text{ cm}^2$$

$$I_{\text{sec}} = (2) (4) (4.9) (18.75)^2 = 13\,800 \text{ cm}^4$$

$$EI = \text{largest} = \left\{ \begin{array}{l} \frac{0.2 E_c I_s + E_s I_{se}}{1 + \beta_{ds}} = \frac{(0.2)(248.7)(470\,000) + (2000)(13\,800)}{1 + 0.6} = 31.86 \times 10^6 \text{ t.cm}^2 \\ \frac{0.4 E_c I_s}{1 + \beta_{ds}} = \frac{(0.4)(248.7)(470\,000)}{1 + 0.6} = 29.2 \times 10^6 \text{ t.cm}^2 \end{array} \right.$$

$$EI = 31.86 \times 10^6 \text{ t.cm}^2$$

$$P_{cr} = \frac{\pi^2 EI}{(KL)^2} = \frac{\pi^2 (31.86)(10^6)}{((0.78)(745))^2} = \frac{\pi^2 (52.37) 10^6}{(745)^2} = 930 \text{ t}$$

$$\alpha = \frac{P_u}{0.75 P_{cr}} = \frac{400}{(0.75)(930)} = 0.573$$

$$S_{ns} = \frac{C_m}{1 - \alpha} = 1.7 < 1.4 = \text{upper limit}$$

$$S_{ns} = 1.4$$

$$M_{\text{max}} = S_{ns} M_m$$

$$= (1.4)(75.6) = 105.84 \text{ t.m}$$

$$\text{check for } e = \frac{P_u}{M_{\text{max}}} = 26.46 \text{ cm}$$

$$P_n = 400 \text{ t} \Rightarrow P_n = \frac{400}{0.65} = 615.4 \text{ t}$$

$$e_{balanced} = 2.$$

$$P_{balanced} = 2.$$

$$X = \frac{0.003}{0.003 + 0.0021} (d) = 25.74 \text{ cm}$$

$$\epsilon_s' = 0.00227 > \epsilon_y$$

∴ Steel in Comp Yield

$$T = (4)(4.9)(4.2) = 82.32 \text{ t}$$

$$C_s = (4)(4.9)(4.2 - 0.85(0.28)) = 77.66 \text{ t}$$

$$C_c = (0.85)(0.28)(0.85)(25.74)(45) = 234.32 \text{ t}$$

$$\sum F_y = 0 \Rightarrow P_b = C_c + C_s - T = 229.66 \text{ t}$$

$$\sum M_{RC} = 0 \Rightarrow P_b(e_b) = (18.75)(T + C_s) + C_c(25 - \frac{0.85X}{2})$$

$$e_b = 27.4 \text{ cm.}$$

$$f_c' = 28 \text{ MPa}$$

$$f_y = 420 \text{ MPa}$$

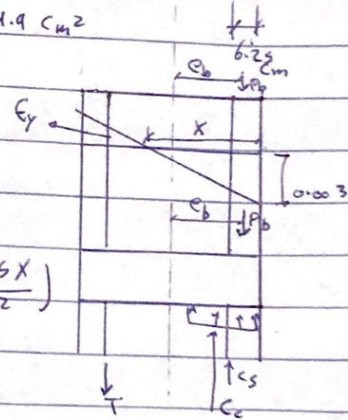
$$\beta_1 = 0.85$$

tie $\phi 10$

$$A_{bar} = 4.9 \text{ cm}^2$$

$$H = 50 \text{ cm}$$

$$B = 45 \text{ cm}$$



$$e = 26.46 \text{ cm}, P_n = ?$$

Compression Control

assume ϵ_s - Steel intension + Not Yield.

Steel in Compression will Yield

$$T = (4)(4.9)(f_s), f_s = \frac{262.5}{X} - 6$$

$$T = \frac{5145}{X} - 117.6$$

$$C_s = (4)(4.9)(4.2 - 0.85(0.28)) = 77.66 \text{ t}$$

$$C_c = 9.1 X$$

$$P_n = C_s + C_c - T = 9.1 X + 195.26 - \frac{5145}{X}$$

$$\sum M_{PC} = 0 \Rightarrow P_n(e) = 18.75(T + C_s) + C_c[25 - \frac{0.85X}{2}]$$

$$241 X + 5167 - \frac{136136}{X} = -749 + \frac{96469}{X} + 227.5 X - 3.87 X^2$$

$$3.87 X^2 + 13.5 X + 5916 - \frac{232605}{X} = 0 \Rightarrow X = 26.11 \text{ cm}$$

check assumption $\epsilon_s = \frac{262.5}{X} - 6 = 4.05 \epsilon_{ky} < \epsilon_y$ ∴ The assumption is correct.

$$\Rightarrow P_n = 235.8 \text{ t} \Rightarrow \phi P_n = 153 \text{ t} < P_u = 400 \text{ t}$$

∴ The Column is not adequate.

