

# Chapter 5: Failures Resulting From static loading

## Static load

Def: stationary force or couple moment applied to a member

- No magnitude change
- point to point application

فشل يحدث في نقطة واحدة فقط stress

We consider relation between Strength and static load (max stress)

### Static Design Criteria

#### Factor of safety (n)

$n = \frac{\text{Strength}}{\text{Stress}} \quad \therefore n > 1$  • this will insure that part won't fail

n can't be negative

#### Reliability approach

$0 < R < 1$  : select the material, processing & dimension such that the probability of failure is less than a preselected value

Ex:  $R = 0.9 \rightarrow 90\%$  chance that the part will function without failure.

#### Types of materials:

- Ductile material :  $C_p > 0.05$ 
  - Design limit
  - yield strength for Tension  $\approx$  yield strength for compression

$S_{yt} = S_{yc} = S_y$  (strength)

2- Brittle material:  $\epsilon_p < 0.05$

No. yield strength:  $S_{uc} > S_{ut}$   
Ultimate strength is our design limit  
Ultimate tensile is less than compression ultimate strength

$$S_{uc} > S_{ut}$$

- Difference: Brittle material break suddenly with no elongation but Ductile material is subjected to necking and elongation

Ductile material

→ Yield criteria

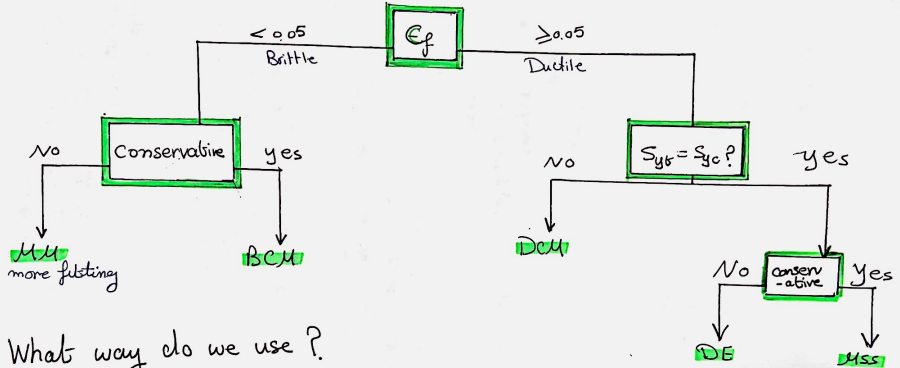
- Max shear stress (MSS) :- Theory 1
- Distortion energy (DE) :- Theory 2
- Ductile Coulomb law (DCM) :- Theory 3

Brittle material

→ Fracture criteria

- Max Normal Stress (MNS)
- Brittle Coulomb Mohr (BCM)
- Modified Mohr (MM)

# Material



What way do we use?

## • Ductile materials

- more accurate  
→ DE (Precise)
- if  $\delta_b \neq S_c$   
use DCM

- $n_{MSS} < n_{DE}$
- MSS is easy  
Quick &  
conservative

# Max Shear stress Theory

$$\tau_{max} = \frac{\sigma_1 - \sigma_3}{2} \geq \frac{S_y}{2} \Rightarrow \sigma_1 - \sigma_3 \geq S_y \text{ (yield strength)}$$

From Mohr circle

$S_y$ : yield strength (comp or tension)  
 $S_{sy}$ : yield strength (shear)

$$n = \frac{S_u}{\sigma_1 - \sigma_3}$$

$$\tau_{max} = \frac{S_u}{2n}$$

## Cases:-

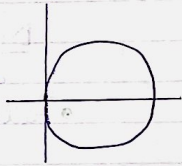
- $\sigma_1$  highest
  - $\sigma_2$  less (middle)
  - $\sigma_3$  lowest
- } Absolute values

### Case 1

Define  $\sigma_1 = \sigma_A$ ,  $\sigma_2 = \sigma_B$ ,  $\sigma_3 = 0$

$\sigma_A \geq \sigma_B \geq 0$ ; we take  $\sigma_A$  as the limit

Tension  $\sigma_A \geq S_y \Rightarrow n = \frac{S_u}{\sigma_A}$

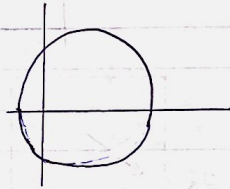


### Case 2

Define  $\sigma_1 = \sigma_A$ ,  $\sigma_3 = \sigma_B$

$\sigma_A \geq 0 \geq \sigma_B$  (negative)

$$\sigma_A - \sigma_B \geq S_y \Rightarrow n = \frac{S_u}{\sigma_A - \sigma_B}$$



### Case 3

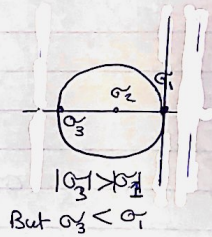
$0 > \sigma_A \geq \sigma_B$

Define  $\sigma_1 = 0$ ,  $\sigma_3 = \sigma_B$

Comp  $\sigma_B \leq -S_y \Rightarrow n = \frac{-S_y}{\sigma_B}$   
 negative  $\rightarrow \sigma_3$

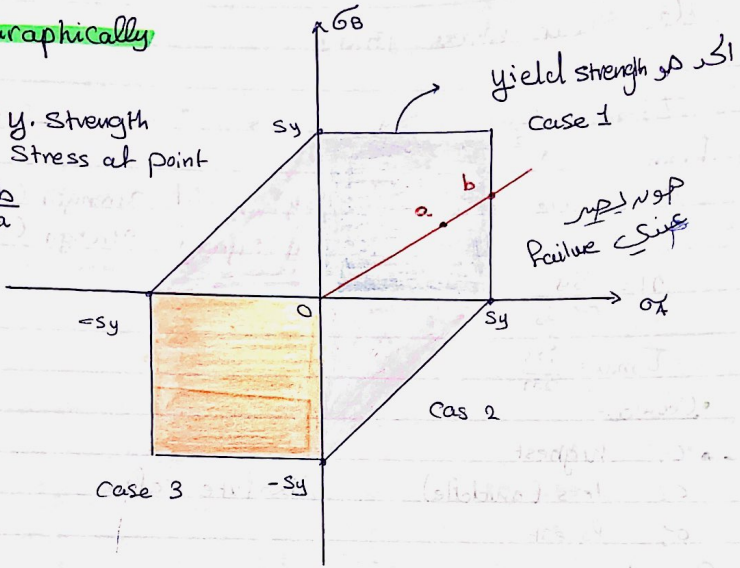
$n \rightarrow$  positive

We take absolute values



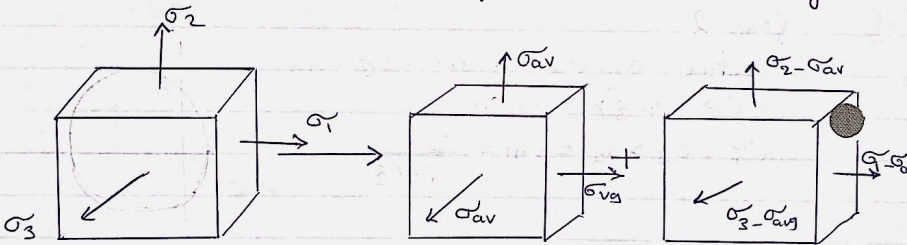
n Graphically

$\sigma_b = y. \text{ strength}$   
 $\sigma_a = \text{Stress at point}$   
 $n = \frac{\sigma_b}{\sigma_a}$



Distortion Energy = theory  $\sigma = \frac{2}{3}$

- Failure occurs when  $\rightarrow$  Distortion strain energy / unit volume  $\geq$  D.S.E / unit volume for yield



Hydrostatic

- stress is acting equally at each face

Distortional

- Not equal

$$\sigma_{av} = \frac{\sigma_1 + \sigma_2 + \sigma_3}{3}$$

$$\sigma_1 > \sigma_2 > \sigma_3$$

Distortion energy when  $\sigma_1 = \sigma_2 = \sigma_3 = 0$  (which we want)  
(No Distortion)

Stress in this Theory is:- (Von Mises Stress)

$$\sigma' = \left( \frac{(\sigma_1 - \sigma_2)^2}{2} + \frac{(\sigma_2 - \sigma_3)^2}{2} + \frac{(\sigma_3 - \sigma_1)^2}{2} \right)^{\frac{1}{2}} \quad \text{if 3D}$$

$$\sigma' = \left( \sigma_x^2 - \sigma_x \sigma_y + \sigma_y^2 + 3\tau_{xy}^2 \right)^{\frac{1}{2}} \quad \text{if 2D } \sigma_z = \sigma_x, \sigma_z = \sigma_y$$

• If I have  $\sigma_x, \sigma_y, \tau_{xy}$  ( $\sigma_x + \sigma_z, \tau_{yz}, \tau_{zx}$  if 3D)  
Then we can use:-

$$\sigma' = \frac{1}{\sqrt{2}} \left[ (\sigma_x - \sigma_y)^2 + (\sigma_y - \sigma_z)^2 + (\sigma_z - \sigma_x)^2 + 6(\tau_{xy}^2 + \tau_{yz}^2 + \tau_{zx}^2) \right]^{\frac{1}{2}} \quad (3D)$$

$$\sigma' = \left( \sigma_x^2 - \sigma_x \sigma_y + \sigma_y^2 + 3\tau_{xy}^2 \right)^{\frac{1}{2}}$$

$$n = S_{sy} / \sigma'$$

• There are 6 points at which  
DE Theory = MSS Theory

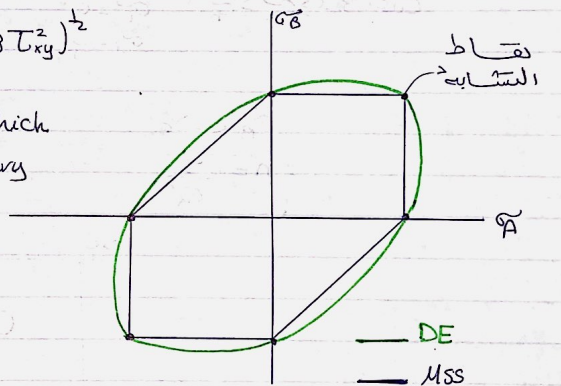
• Pure shear stress:-

$$\sigma' = S_y = \left( 3(\tau_{xy})^2 \right)^{\frac{1}{2}}$$

$$\Rightarrow \tau_{xy} = \frac{S_y}{\sqrt{3}} = 0.577 S_y$$

$S_{sy} = 0.577 S_y$  in Distortion energy theory this is  
The shear yield strength

(In MSS  $S_{sy} = 0.5 S_y$ )



$n_{DE} > n_{MSS}$   
So more conservative

# Coulumb Mohr Theory:-

MSS is allowed

- Difference here is that  $S_t \neq S_c$

## Cases:

Case 1:-  $\sigma_x \geq \sigma_y \geq 0$  ,  $\sigma_1 = \sigma_x$  ,  $\sigma_3 = 0$   
 $\Rightarrow \sigma_x \geq S_t \Rightarrow n = \frac{S_t}{\sigma_x}$

Case 2:-  $\sigma_x \geq 0 \geq \sigma_y$  ,  $\sigma_1 = \sigma_x$  ,  $\sigma_3 = \sigma_y$

$$\frac{\sigma_x}{S_t} - \frac{\sigma_y}{S_c} \geq 1$$

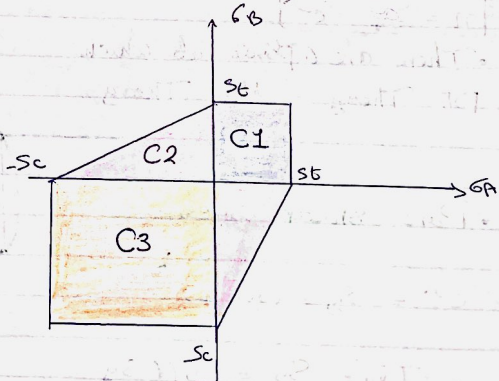
$$\Rightarrow n = \frac{\sigma_x - \frac{\sigma_y S_t}{S_c}}{S_t}$$

we have to divide because Triangle does not have equal

Case 3:-  $0 \geq \sigma_x \geq \sigma_y$  ,  $\sigma_1 = 0$  ,  $\sigma_3 = \sigma_y$

$$\sigma_y \leq -S_c$$

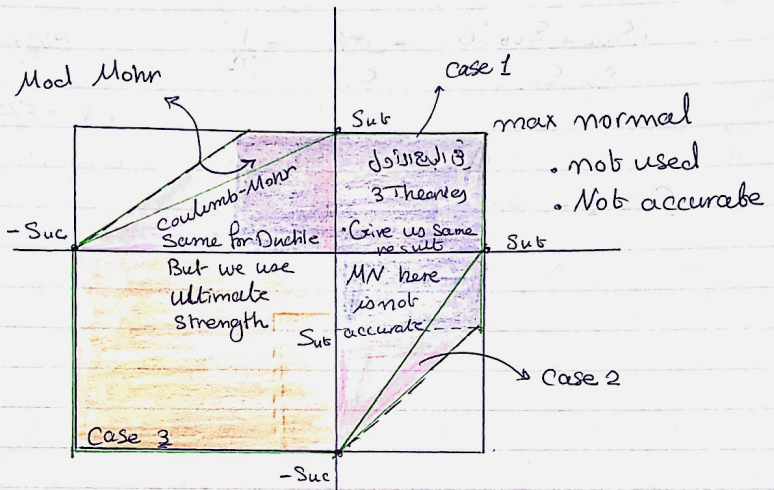
$$n = \frac{S_c}{\sigma_y}$$



To find  $S_{sy}$ : ( $\tau_{max}$ )

$$\tau_{max} = S_{sy} = \frac{S_{yt} S_{yc}}{S_{yt} + S_{yc}}$$

## Brittle material



- Coulumb's Theory :-

$$\sigma_A = \frac{S_{ub}}{n} \quad \sigma_A \geq \sigma_B \geq 0$$

$$\frac{\sigma_A}{S_{ub}} - \frac{\sigma_B}{S_{uc}} = \frac{1}{n} \quad \sigma_A \geq \sigma_B$$

$$\sigma_B = -\frac{S_{uc}}{n} \quad 0 \geq \sigma_A \geq \sigma_B$$

- Modified Mohr

Case 1 :-  $\sigma_A = \frac{S_{ub}}{n} \rightarrow \sigma_A \geq \sigma_B \geq 0$

$$\text{or } \rightarrow \sigma_A \geq 0 \geq \sigma_B, \quad \left| \frac{\sigma_B}{\sigma_A} \right| \leq 1$$



Case 2:-

$$\frac{(S_{uc} - S_{ub})\sigma_A}{S_{uc} S_{ub}} - \frac{\sigma_B}{S_{uc}} = \frac{1}{n}$$

للمرجع الثاني والرابع

$$\sigma_A \geq \sigma_B \quad \left| \frac{\sigma_B}{\sigma_A} \right| < 1$$

لندعم التنبؤ على هذا الشرط.

Case 3:-  $\sigma_B = -\frac{S_{uc}}{n}$

$$0 \geq \sigma_A \geq \sigma_B$$